

Standard Penetration Test with Torque and Weak Rock Procedure

Benefits of the Standard Penetration Test:

- Can almost always advance the boring to the desired depth in overburden soils
- Obtain a soil sample for visual classification and laboratory tests
- Torquing the sampler after driving provides a static strength parameter and predicts side resistance for deep foundations
- In weak or weathered rock, Stark et al. (2021) measures the penetration distance for every 10 blows (modified test procedure [MSPT]) to evaluate the unconfined compressive strength (UCS) between 10 and 100 ksf.

While the standard penetration test is probably the most common in-situ test performed in North and South America, the term “standard” misleads design engineers. Although the test is relatively simple to perform, only skilled drillers routinely achieve meaningful results. Terzaghi and Peck (1948) published early geotechnical design correlations, such as the spread footing design chart for sand shown as Figure 1, which popularized the SPT and encouraged its acceptance as a "standard". McGregor and Duncan (1998) updated the design correlations and testing procedure for the SPT. Lutenege (2021) recommends procedures that should be adopted by every geotechnical engineer when performing SPT.

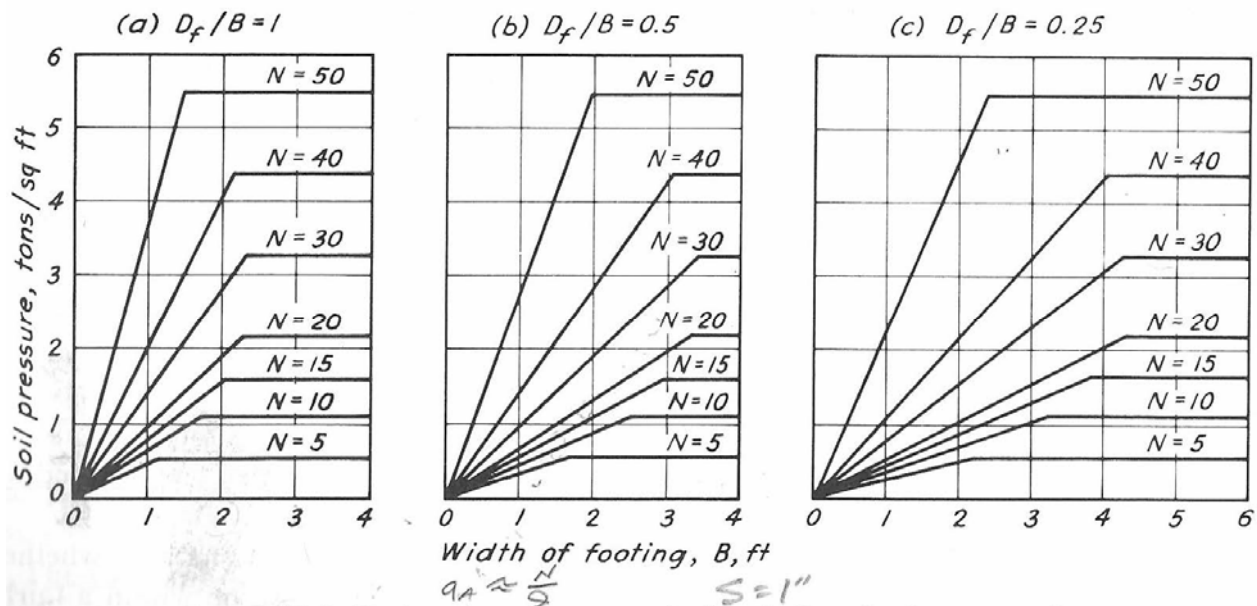


FIGURE 19.3. Design chart for proportioning shallow footings on sand.

Figure 1: Peck design chart for spread footings on sand

History of the Standard Penetration Test (SPT), ASTM D1586, D4633, and D6066: In 1902, C.R. Gow designed a 1-inch diameter heavy-wall sampler to be driven with a 110 pound weight. In 1927, L. Hart and G.A Fletcher developed the standard 2-inch-diameter "split-spoon" sampler (Figure 2). Later, Fletcher and H. A. Mohr standardized the test using a 140-pound hammer with a 30-inch drop to measure the blow count for three consecutive 6-inch increments of penetration, reporting the total blow count for final 12 inches as the N_{SPT} value. GeoProbe manufactures a finger interlocking split spoon sampler (Figure 3) that the driller can take apart and assemble more quickly than a traditional split spoon sampler. Their locking system prevents the sampler seams from bowing outward.



Figure 2: Split spoon SPT sampler



Figure 3: Finger Split Spoon Sampler [Left: opened with sample] [Right: assembled]

The three styles of SPT hammer in common use (see Figure 4) deliver energy to the drill rods that varies from about 45% to 95% of the theoretically available potential driving energy of 4200 in-lbs (140 pounds x 30 inch drop). This variation, plus the use of non-standardized drilling techniques, led [Schmertmann \(1978\)](#) to investigate their effect on the value of N_{SPT} , which he found to exceed a factor of two. In addition, [Schmertmann \(1979\)](#) also found that N_{SPT} varied approximately inversely in proportion to the hammer energy delivered to the drill rods. [Schmertmann and Palacios \(1979\)](#) made energy measurements comparing operator delivered energy with a hammer raised to 30 inch height and cutting the lifting rope, simulating free fall hammer drop. Now with the advent of modern computers, energy measurement devices (Figure 5) allow engineers to easily measure the actual driving energy entering the rods as described in ASTM D4633. The engineer can then correct the measured value of N_{SPT} to N_{60} , the equivalent blow count at 60% of the theoretical hammer energy (thought to represent the average energy in the correlation database). [Skempton \(1986\)](#) presented a method to compute N_{60} values from raw N_{SPT} data, which is incorporated in ASTM D6066.

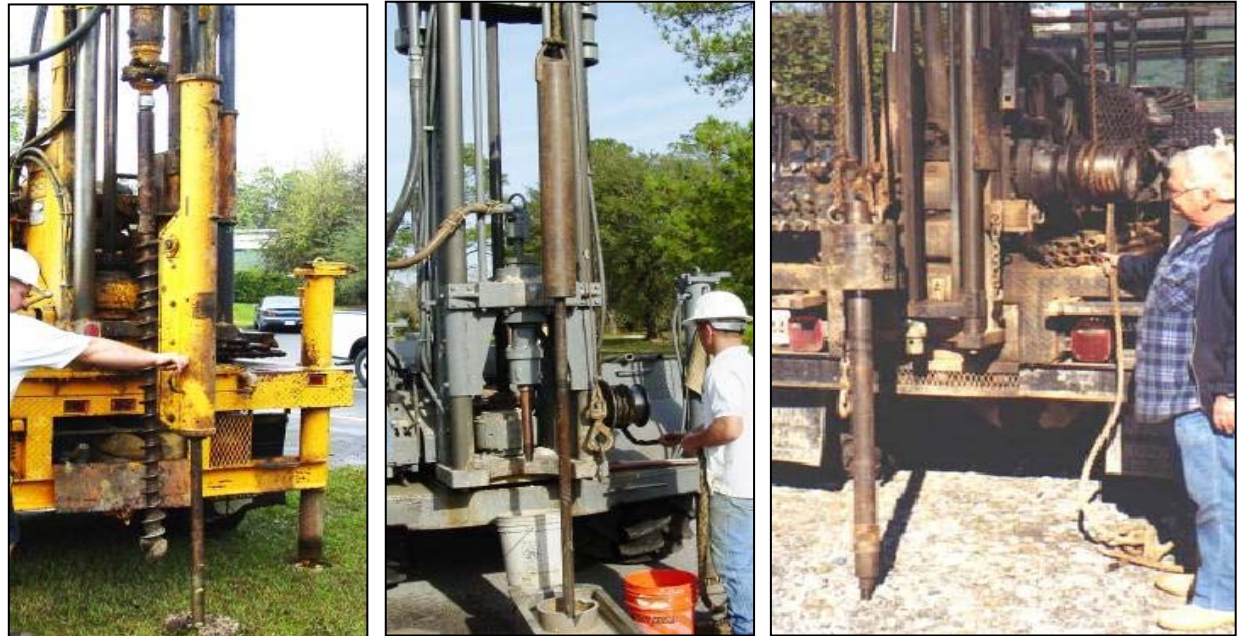


Figure 4: a) Automatic Hammer ~95% efficiency,
 b) Safety Hammer ~60% efficiency,
 c) Donut Hammer ~45% efficiency.
 (photo from GeoServices Corp.)

Figure 4: The three styles of SPT hammer in common use

Unfortunately, N_{60} values rarely appear on boring logs. The barrel on the old samplers had the same inner diameter as the shoe (1.375 inch 35 mm ID). Today, an alternative sampler barrel in common use has a larger inside diameter (1.5 inch 38 mm ID) to accommodate liners with an inner diameter the same as the shoe. However, liners are rarely used - Skempton suggests multiplying the N -value by 1.2 for this correction to account for the lack of frictional resistance along the inside of the split spoon sampler. Automatic trip hammers, now in widespread use, may deliver almost 95% of the theoretical energy if well-maintained. For these hammers, the engineer multiplies N_{field} by 1.58 to get N_{60} . The engineer can convert the field values to N_{60} values using this Excel spreadsheet [SPT N field value to \$N_{60}\$ Look-up Table](#). Without making the N_{60} correction results in overly conservative and costly design.

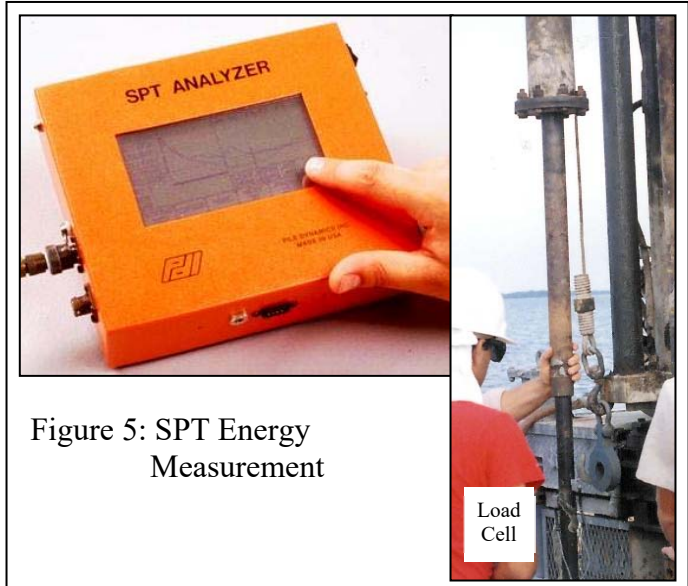


Figure 5: SPT Energy Measurement

The dynamic energy delivered from the SPT hammer to the split spoon sampler remolds sensitive clay and destroys the latent rock structure found in residual soil. Based on these low SPT N values, engineers' predictions of deformation modulus from standard correlations give moduli that are about $\frac{1}{4}$ of their actual values (from our experience at various project sites). Even with the best techniques, how the soil responds to static structural loading based on the results of a dynamic test penetration tests leads to highly inaccurate settlement predictions for embankments or shallow spread footings.

SPT models the driving of a pile. [Schmertmann and Crapps \(1983\)](#) show that the SPT based on N_{60} values better predicted the vertical capacity of piles than other design methods using different tests. [DeCourt \(1998\)](#), [Lutenegger and Kelley \(1998\)](#), and [Kelley and Lutenegger \(1999\)](#) show better predictions of side resistance for driven piles and generally better predictions of soil properties with the addition of a torque measurement of the split spoon sampler after driving it. Rotating the split spoon sampler at a constant rotation rate of 0.2 degrees per second using an electric motor removes testing error from manually rotating the spoon (Figure 6).



Figure 6: Electric motor SPT torque equipment (left) Top adapter for hoist cable and during test for pipe wrench to provide reaction for torque motor (right)

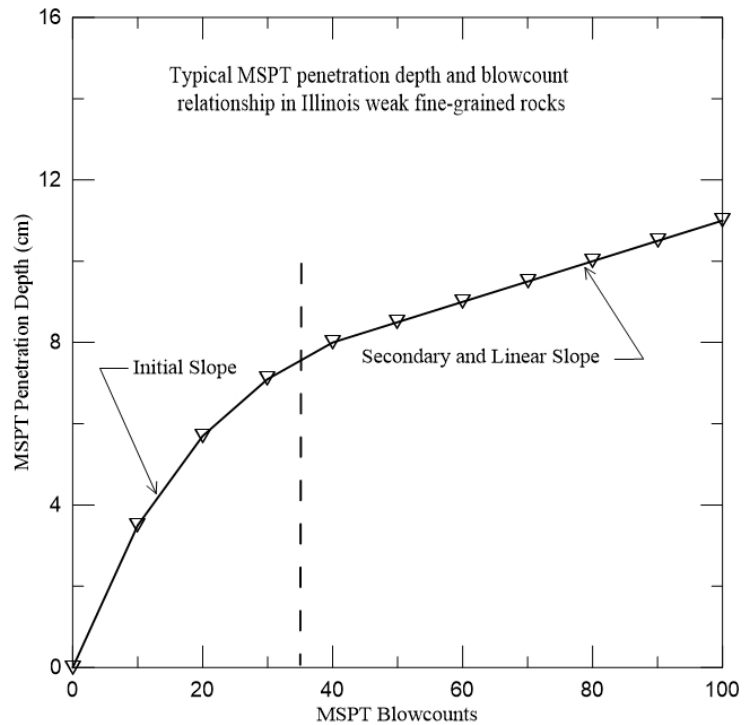
Shown on the adjacent [video](#) the engineer accurately measures the peak torque value three minutes after the spoon was driven. If the split spoon remains in the soil overnight and then torqued the following morning, then the engineer can get a measure of pile set-up. Additional research comparing load tests and SPT-Torque measurements will improve designs in the future.

[Stark, et. al. \(2017\)](#) modified the standard penetration test procedure (MSPT) using calibrated energy (MSPT) to assess the in situ unconfined compressive strength (UCS) and Young's modulus of weak fine-grained rock for design. He termed this new procedure the Modified SPT (MSPT) and measured the penetration for every 10 hammer blows to 100 blows (10

measurements) or when penetration ceases to predict UCS. With the MSPT, the inspecting geologist or engineer makes the additional penetration measurement after every ten hammer blows, while the driller makes the standard SPT blow count measurement.

Figure 7 shows the penetration depth versus blow count relationship, characterized by an *initial slope* and a *secondary slope*. The *initial slope* is associated with disturbed and loose material at the bottom of the borehole and the tip of the split-spoon sampler easily penetrating this material. The *initial slope* is displacement or depth dependent and shows displacement hardening behavior. The *initial slope* does not represent the UCS of the undisturbed or intact weak rock and thus is not used for the \dot{N}_{90} (N_{Rate})₉₀ correlation developed herein.

Figure 7: Typical MSPT penetration depth versus blow counts plot for Illinois weak shale



The *secondary or linear slope* represents the undisturbed or intact weak rock, which usually occurs after 30 blows, and is approximately constant with increasing displacement. Stark defines the slope of that line as the \dot{N}_{90} (N_{Rate})₉₀ for a hammer with an energy efficiency of 90%. For a hammer with an efficiency different than 90%, the engineer multiplies the calculated slope by the hammer's calibrated efficiency/90%. The engineer uses this computed slope to develop a site or project specific correlation with the unconfined compressive strength (UCS) of the undisturbed weak rock and/or verifies the correlation developed by Stark et. al. applies. Based on a subsurface investigation at twenty-one (21) weak fine-grained shale/siltstone sites throughout Illinois, Stark, et. al. (2021) developed the following correlations between \dot{N}_{90} and UCS to obtain side resistance (Figure 8) and end bearing (Figure 9) of deep foundation systems.

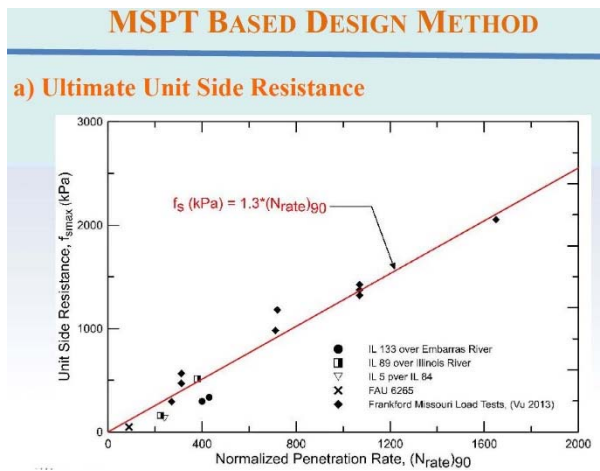


Figure 8: Frictional Resistance versus N90 rate (Weak Shale)

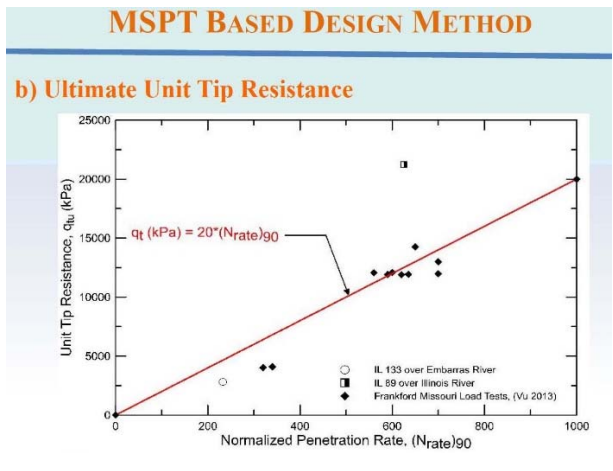


Figure 9: Tip Capacity versus N90 rate (Weak Shale)

Figure 10 shows the average trend line can be expressed as UCS (q_u) equals:

$$q_u \text{ (kPa)} = 4.4 (N_{Rate})_{90}$$

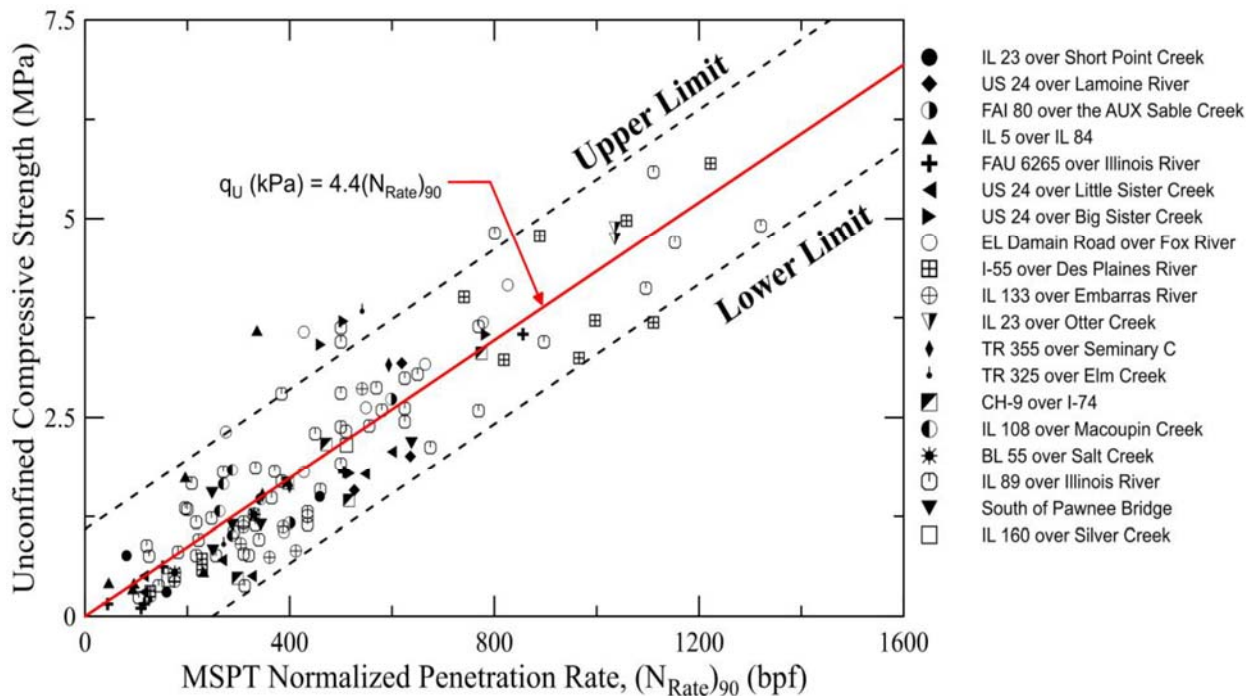


Figure 10: Relationship between UCS and N_{90} (N_{Rate})₉₀ for weak shale in Illinois

Stark suggests using the following MSPT procedure and compute the N_{90} (N_{90} rate) [MSPT Video]:

1. Drill to the desired depth of the MSPT,

2. The split-spoon sampler and the driving shoe should be in good to new condition and must be replaced when it becomes dented or distorted.
3. Choose and mark a convenient point on the exposed drill rod at which penetration measurements will be made; such as the bottom of the anvil or a drill rod joint or third mark for SPT.
4. Measure the initial length of the drill rod segment between the top of the borehole casing and the point chosen in Step 3.
5. Apply ten (10) blows to the top of the drill rod using the 63.5 kg (140 lbf) hammer falling 0.76 m (30 inches) and measure the new length of the drill rod segment between the top of the borehole casing and mark/point chosen in Step 3. This can be accomplished by stopping the test or using a stick rule that is placed along this length and reading the remaining length before the 11th blow is applied by the automatic hammer.
6. Repeat Steps 3 through 5 to obtain the sampler penetration for the 20-, 30-, 40-, 50-, 60-, 70-, 80-, 90-, and 100-blow count increments.
7. Input the penetration depth versus the cumulative blow counts using [Stark's Excel spreadsheet](#) to compute \dot{N}_{90} ($N_{Rate}90$) and UCS of the weak rock.

Figure 11 presents typical MSPT test results.

Measured Rod Length (ft)	Blows where exposed rod length was measured											• N (bpf)	q _u (ksf)	Young's Modulus (ksi)
	0	10	20	30	40	50	60	70	80	90	100			
394.50	3.24	3.00	2.82	2.64	2.52	2.42	2.32	2.23	2.14	2.04	1.96	104.9	6.6	1.57
389.50	3.28	3.09	2.97	2.90	2.85	2.81	2.78	2.73	2.69	2.66	2.63	259.0	18.2	4.07
384.50	3.28	3.06	2.91	2.79	2.70	2.62	2.54	2.48	2.42	2.36	2.31	172.4	11.5	2.63
379.50	3.27	3.16	3.09	3.04	3.00	2.96	2.94	2.92	2.91	2.90	2.89	833.3	67.2	18.60
374.50	3.29	3.20	3.13	3.07	3.04	3.01	2.98	2.96	2.94	2.93	2.91	588.2	45.5	11.13
369.50	2.90	2.79	2.74	2.70	2.66	2.64	2.62	2.61	2.59	2.58	2.57	618.7	48.2	11.97
364.50	2.92	2.82	2.77	2.72	2.71	2.69	2.67	2.65	2.64	2.63	2.62	601.6	46.7	11.50
359.50	2.95	2.85	2.80	2.76	2.73	2.71	2.69	2.68	2.66	2.65	2.64	618.7	48.2	11.97
354.50	2.99	2.80	2.71	2.62	2.53	2.45	2.38	2.31	2.27	2.21	2.16	173.4	11.6	2.66
349.50	3.00	2.88	2.82	2.78	2.76	2.73	2.72	2.70	2.68	2.67	2.67	500.0	37.9	8.89
344.50	3.00	2.85	2.79	2.76	2.75	2.73	2.72	2.71	2.70	2.70	2.70	1721	151.5	102.38
339.50	3.06	2.96	2.96	2.95		2.93		2.90	2.90	2.88	2.88	909.1	74.1	21.27

Note: **Bolded** values used in calculating \dot{N} .

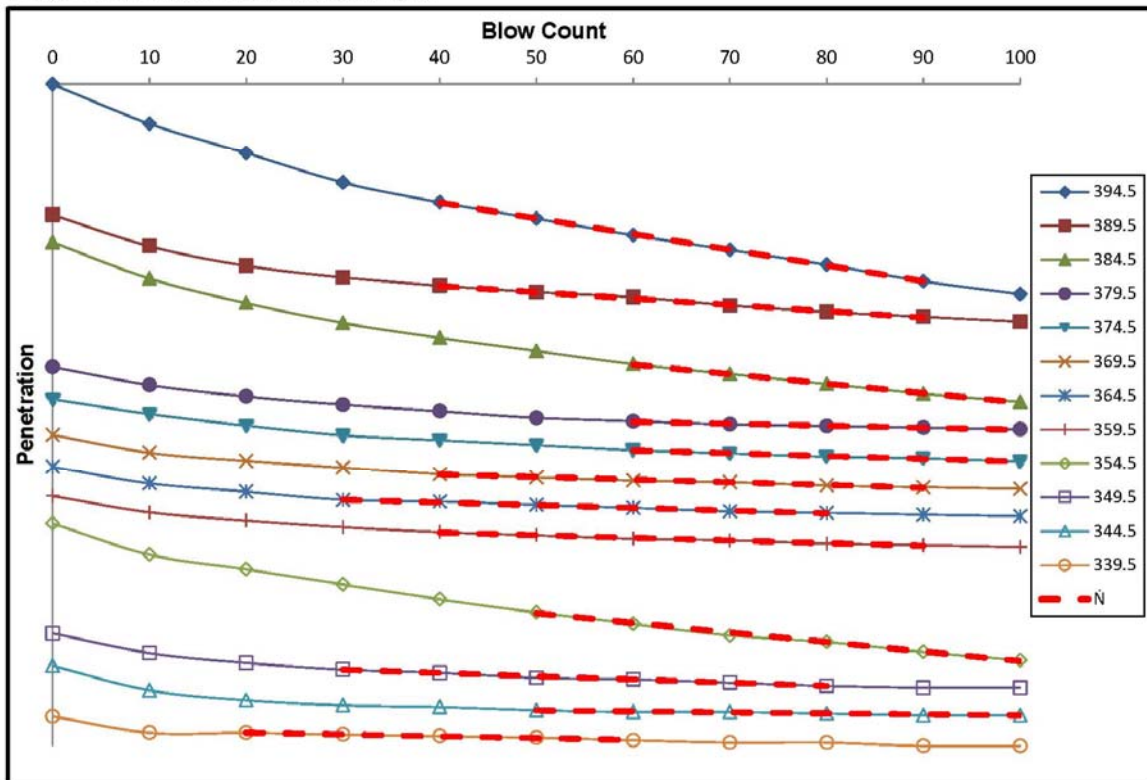


Figure 11: Typical modified SPT results